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March 6, 2020

Halifax Water  
 450 Cowie Hill Road  
 Halifax, NS  
 B3P 2V3

From: Alex Pulsifer, P.Eng.

File No. 1-3-62 (34976)

**Re: 13 Unit Residential Development, 48/50 Old Sambro Road, Halifax, NS –  
 Sanitary Lateral Size Confirmation**

**Project Summary:**

|                 |       |
|-----------------|-------|
|                 | Units |
| Residential     | 13    |
| *From Developer |       |

**References:**

- Halifax Water (HW) Design & Construction Specifications (2018 Edition), Section 4.2.2:

- $Q = [1.25 \times (a \times M)] + b$       Where;
  - Q = Sanitary sewer flow.*
  - 1.25 = Safety factor.*
  - a = Average dry weather flow.*
  - M = Peaking factor using Harmon Formula;  $M = 1 + [14 / (4 + P^{0.5})]$*
  - b = Long-term infiltration/inflow allowance.*
  - P = Population in thousands*

- Residential Average Dry Weather Flow: 300 L/day per person
- Multi-unit dwellings: 2.25 people per unit
- Infiltration allowance: 0.28 L/ha<sub>gross</sub>/s

**Calculation Summary:**Population Estimate (P)

Reference:

HW Section 4.2.1 Residential (Multi-unit dwellings): 2.25 people per unit

$$P = 2.25 \text{ people per unit} \times 13 \text{ Units} = \mathbf{30 \text{ people (or 0.030)}}$$

Dry Weather Flow (a)

Reference:

HW Section 4.2.2: Residential: 300 L/day per person

$$a = 300 \text{ L/day per person} \times 30 \text{ people} = \mathbf{9,000 \text{ L/day (or 0.104 L/s)}}$$

Infiltration (b)

Reference:

HW Section 4.2.2: Infiltration allowance: 0.28 L/ha<sub>gross</sub>/sLot Area = 1195 m<sup>2</sup> = 0.12 ha

$$b: \quad 0.28 \text{ L/ha}_{\text{gross}}/\text{s} \times 0.12 \text{ ha} = \mathbf{0.034 \text{ L/s}}$$

Peaking Factor (M)

$$M = 1 + [14 / (4 + P^{0.5})]$$

$$M = 1 + [14 / (4 + (0.030)^{0.5})] = \mathbf{4.35}$$

Sanitary Sewer Flow (Q)

$$Q = [1.25 \times (a \times M)] + b$$

$$Q = [1.25 \times (0.104 \text{ L/s} \times 4.35)] + 0.034 \text{ L/s} = \mathbf{0.60 \text{ L/s}}$$

**Sanitary Lateral Size Confirmation:**

A 150mm diameter PVC lateral at 2.00% slope has a capacity of 28.0 L/s. With Q = 0.60 L/s, the proposed lateral will have sufficient flow capacity. For additional information or discussion regarding these findings please contact the undersigned.

Regards,

**Servant, Dunbrack, McKenzie & MacDonald Ltd.****Original Signed**

Alex Pulsifer, P.Eng.  
Project Engineer